

GENERAL STATEMENT OF WORK

The owner would like to build a new garage addition to his existing residence located at 320 Melbourne Avenue in Indialantic. The garage walls will be constructed of cement block with a wall height of 16'-8". The roof will be constructed using pre-engineered roof trusses supporting metal roofing.

ENGINEERING NOTES

1.) These plans are certified to be designed in accordance with the 2023 Florida Building Code, Residential Eighth Edition; and the 2023 Florida Building Code, Building, Eighth Edition; 2023 Florida Building Code, Existing Building, Eighth Edition, ASCE 7-16, and ASCE 7-22 using an ultimate wind load "3 second gust velocity" as follows:

DESIGN CRITERIA

Wind Speed = 160 MPH (3 sec. gust)
Risk Category = II, (2)
Exposure Category = C
Enclosure Category. = enclosed
Internal pres. coef. + 0.18 &- 0.18
Importance factor = 1.0

DESIGN LOADS

Soil Bearing Pressure - 2,000 psf (see below)
Floor loads - live load 60 psf
Roof Trusses (Minimum design loads)
Top Cord: live load= 20 psf, dead load= 15 psf
Bottom Cord dead load = 10 psf

ULT / ASD Load Equivalents

160 mph "ultimate" = 124 mph "allowable" (nominal) FBC TABLES R301.2.1.3 & 1609.3.1
Design Pressures = DP(ult) x 0.60 = DP(asd) FBC SECTIONS R301.2.1.3 & 1609.3.1

VELOCITY PRESSURE = $q_z = 0.00256 K_z K_{zt} K_d V^2 = 52.36 \text{ PSF}$
at mean roof height of 30 feet.

The minimum window & door design pressures are specified under "DESIGN LOAD PRESSURES FOR WINDOWS AND DOORS" in these Notes. This Engineering Design is for resisting wind loads on this particular building to be built on this specific site and cannot be used for any other building or structure.

2.) FOUNDATIONS AND SOIL

This plan assumes virgin soil under the building area plus 5 feet beyond. A Geotechnical Engineering and Testing Company should be retained to perform soil borings and make foundation recommendations prior to beginning any work. The foundations shown on these plans have been designed for an allowable soil bearing pressure of 2000 PSF. Soil borings are not required by the building department, however, we recommend these tests to avoid long term foundation settlement. If the owner chooses not to obtain soil borings, then the owner assumes all responsibility for abnormal foundation and slab settlement in the future.

The soil directly beneath the buildings and 5 feet beyond should be prepared in accordance with your geotechnical engineers recommendations, and/or the following (whichever is more stringent). All fill shall be placed and compacted in 12" lifts (max). Compaction shall be tested beneath foundations and floor slabs and shall have a minimum value of 95% as determined by the modified proctor test. The soil beneath foundation & slabs must be tested by an Engineering Testing Lab for compaction prior to pouring concrete.

3.) CONCRETE MATERIALS and PLACEMENT

Foundation and Slab Concrete shall have a minimum "28 day" compressive strength of 3000 psi (4000 psi recommended) and be "regular mix" (3/4" rock) poured at a maximum slump of 5 inches. Block Wall cells and Tie Beams shall be pumped with "pea-rock" pump mix (1/4" rock) with a "28 day" compressive strength of 4000 psi, and a maximum slump of 10".

All wall concrete shall be vibrated (or rodded) as it is poured. Wall concrete may be rodded using the downpour steel. The walls are built in two sections, first construct it to 8' height and poured with #5 steel protruding 25" minimum, then construct the upper wall. All slabs shall be cured with water for a period of 3 days. Use curing compound, soaker hoses, or tarps. Saw-Cut 1.5" deep control joints in all slabs within 12 hours (12' max. each way).

4.) REINFORCING STEEL and PLACEMENT

Reinforcing steel (rebar) shall have a minimum yield strength of 60 KSI (Grade 60). All lap splices shall be a minimum of 25 inches. All bends shall have a minimum radius of 6 bar diameters. All reinforcing steel shall have a minimum exterior concrete cover of 1.5" except that all reinforcing steel used in the foundations shall have a minimum concrete cover of 3". Minimum "interior concrete cover" shall be a minimum of 3/4" for slabs and 1" for interior faces of beam stirrups and column ties.

5.) BLOCK MATERIALS and PLACEMENT

The exterior block walls are 16'-8" and constructed in two sections. First construct it to 8' height and poured with #5 steel protruding 25" minimum, then construct the upper wall. Block Walls shall be constructed using 8" x 8" x 16" hollow "concrete" masonry block with a "net area" compressive strength of 1900 psi (min.) constructed using type M or S mortar meeting ACI Code, sections 530-08 & 530.1-08 and ASTM C 90-08. All block shall be laid in running bond and interlocked at all corners and intersections unless specified otherwise on the floor plans. All block walls shall have downpours where shown on the floor plans. A continuous tie beam shall be provided over all block walls. All exterior walls are masonry block - See specifications in note #5 & Wall Section (this sheet).

All headers are pre-cast lintels with lintel block course above (see typical wall section).

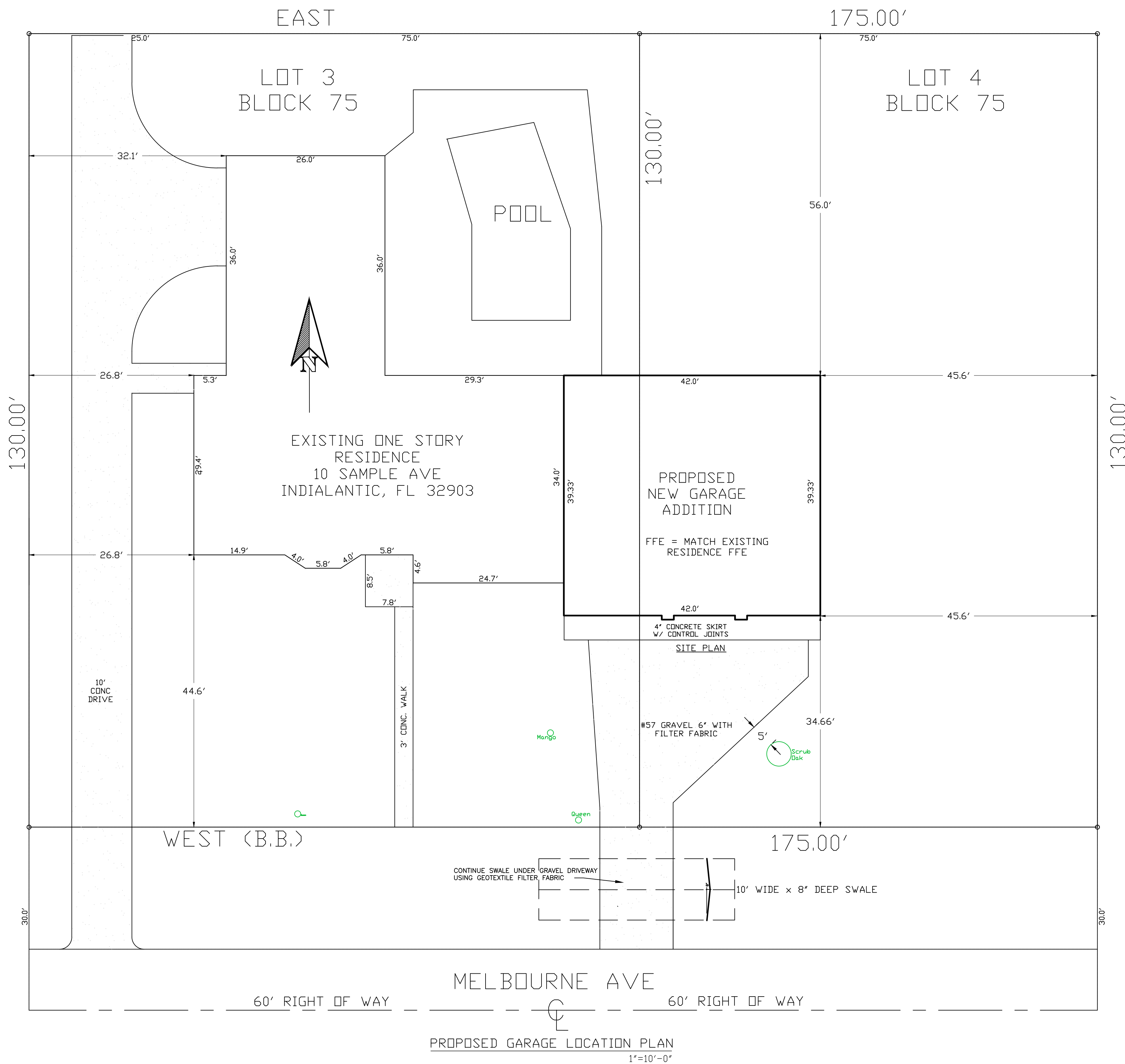
CUT ONE BOTTOM FACE OUT OF ALL DOWNPOUR BLOCK CELLS, WITH A MINIMUM AREA OF 12 SQUARE INCHES AND A MINIMUM DIMENSION OF 3 INCHES, TO CLEAN THE CELL, TIE STEEL, AND TO ENSURE A PROPER POUR.

DOWNPOUR STEEL (where shown on the floor plan) shall be one #5 bar continuous from the foundation to the upper tie-beam. The bars shall be hooked 12" in the foundation (along the bottom steel) and 12" in the upper tie-beam (along the top steel) with 24" Lap splices (Min.)

6.) CONCRETE BEAMS

All block walls shall have two (2) continuous reinforced concrete tie-beams. See Floor Plans and wall section for beam type and location.

Corner Bars - All tie beam reinforcement at corners shall be continuous with two (min) #5 corner bars tied in place prior to pouring concrete. The corner bars shall extend a minimum of 24" into each tie beam.



PROPOSED GARAGE LOCATION PLAN
1"=10'-0"

THIS LOCATION PLAN IS BASED ON THE
1-30-2015 SURVEY OF THE PROPERTY
PROVIDED BY SAMPLE LAND SURVEYING
SERVICES INC.

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ENGINEERING NOTES CONTINUED...

7.) Concrete testing – strength & slump tests – At least one slump test and at least one set of five concrete strength test cylinders shall be made during the foundation and floor slab pour. Air temperature, concrete temperature, and added water shall also be recorded. Concrete cylinders shall be broken in accordance with ASTM standards as follows:

Two cylinders at 7–days, two cylinders at 28–days, one extra – tested at contractors request
Concrete slump limitations: 4.5" min. and 6.5" max. for footings, slabs, beams, columns and walls.
8" to 10" for masonry downpours.

MISSED REINFORCING DOWELS AND CONNECTORS

Reinforcing steel that is misplaced can be drilled and epoxied 10" into concrete using "Simpson" SET–3G (or equal) in accordance with the manufacturers specifications.

Embed hurricane straps that are left out (or misplaced by more than 1") must have a new strap attached to the side of the tie beam with 3/8"x2 1/4" TAPCON (or TITEN screws). Use one Tapcon for each 300 lbs. of uplift and nail strap to truss per manufacturers specifications.

8.) UPLIFT CONNECTORS

Use the following galvanized truss straps (or equal connectors) to resist the truss uplift loads categorized below or use these as an example. Use connector strengths and nailing requirements in accordance with the manufacturers' recommendations.

TRUSSES ON CONCRETE TIE BEAMS:

Up to 1810 lbs uplift – must be single ply truss – One "Simpson" HETAL20 embedded truss straps 6" into the tie beam, with (14) – 10d x 1–1/2" galv. nails.

Up to 2500 lbs uplift – must be double ply truss – two "Simpson" HETAL20 embedded truss straps 6" into the tie beam, with (14) – 10d x 1–1/2" galv. nails. in each.

Up to 8885 lbs uplift – must be triple ply truss – two "Simpson" FGTR girder tie-downs bolted into the tie beam, with four 1/2" x 5" Titen HD Bolts and thirty six 1/4" x 3" SDS screws.

9.) ROOF TRUSSES & ROOF SHEATHING

The roof structure consists of pre-engineered trusses placed 16" o.c. The owner is requesting "Minimum" 2x6 top cord members and 2x6 bottom cord members. The trusses shall be designed and engineered for the wind loads & categories stated in Note #1, by a "truss system" engineer in accordance with the "2023 Florida Building Code, Building, Eighth Edition." Final truss engineering shall include a signed & sealed truss system layout as well as individual drawings for each truss type. Truss drawings shall include wind design information, vertical and horizontal reactions, uplift forces on all trusses, all truss bracing requirements, and specify all girder loads.

The roof trusses shall be sheathed with 5/8 (19/32") CDX plywood (min.) and nailed with 8d (2 1/2") galvanized common "ring–shank" nails at 4" o.c. everywhere within 5'–0" of all roof edges and ridgelines and 3" o.c. within 5'0" of all corners. All other areas can be nailed at 6" on center (staples are not allowed). Install aluminum clips between horizontal plywood joints or "gap" the joint 1/8" (min.). Where truss or roofing manufacturer specifications differ from these, use the more stringent. Brace all trusses as required by the truss manufacturer.

CONVENTIONAL ROOF FRAMING is required to extend existing roof. (See Sheet 5 for details)

10.) CLADDING and COMPONENTS

Siding, roofing, soffits, and all other cladding and components shall be fastened in strict accordance with the manufacture specifications to resist the wind loads & categories stated in Note #1 (this includes decorative attachments). Contractors must make these manufacturers provide certifications that their product and fastener details meet or exceed the requirements of the 2023 Florida Building Code, Building, Eighth Edition for this wind zone. The minimum design pressures are the same as used for windows and doors (see Note #13 on this sheet).

11.) ROOFING MATERIALS and INSTALLATION

The Roofing material manufacturer shall provide product certification showing that the roofing materials and their installation comply with all requirements of the 2023 Florida Building Code, Building, Eighth Edition and the 2023 Florida Building Code, Residential, Eighth Edition for the wind loads & categories stated in Note #1 (exposure C) and with all sections of Chapter 15 of the FBC Building and Chapter 9 of the FBC Residential. And be labeled by the manufacturer showing the manufacturers name, model, or type of product, and ASTM product approval information. The Building Contractor shall verify that the Roofing sub–contractor has a copy of these code sections. These sections are summarized below:

a.) All roofing components shall be rated for 160 mph (exposure C) wind loads in accordance with the 2023 Florida Building Code, Building, Eighth Edition, and be labeled by the manufacturer showing the manufacturers name, model, or type of product, and ASTM product approval information.

b.) Self–adhering polymer modified bitumen sheet complying with the requirements of the manufacturers' product approval sheet, and ASTM/D–1970. Per FBC Eighth Edition "Residential" Section R905.1.1.1, Option 1; the entire roof deck shall be covered with the approved self–adhering polymer–modified bitumen underlayment complying with ASTM D1970 installed in accordance with both the underlayment manufacturer's and roof covering manufacturer's installation instructions for new plywood roof sheathing.

c.) Metal roofing over 5/8" (19/32") CDX plywood and shall comply and be installed in accordance with manufacturers specifications and their product affidavit sheet for 160 mph (exposure C) "124 mph allowable" in accordance with the 2023 Florida Building Code.

d.) Shingle roofing over 5/8" (19/32") CDX plywood and shall comply and be installed in accordance with manufacturers specifications and their product affidavit sheet for 160 mph (exposure C) "124 mph allowable" in accordance with the 2023 Florida Building Code.

12.) CONVENTIONAL ROOF FRAMING TO EXTEND THE ROOF ON THE EXISTING RESIDENCE

Where conventional framing is used to "fill–in" and extend the existing roof, a new 2x10 SYP Ridge Beam shall be installed and connected to the existing ridge beam using 5/8" plywood or metal plates on each side. Connect Ridge Beam to new garage wall using a SIMPSON HUS210 Hanger. Install 2x6 SYP rafters at 16" on center. Connect each Rafter to new ridge beam using a SIMPSON LRU26Z Sloped Hangers.

The new Ridge Beam and the new Rafters shall ALSO be support with 2x4 vertical "Stiff–Legs" members every 6 feet or less. The Stiff–Legs shall line up over existing roof framing members and be strapped at each end using SIMPSON LSTA 12 Straps.

On all members over 36" long – Install HURRICANE STRAPS

Attach base of rafters and each end of other framing members to resist uplift forces equal to 50 lbs. multiplied by the overall rafter/member span length (from stiff–leg).
i.e – A 6'0" long framing member requires a strap rated for 300 lbs.. Members shorter than 36" can be nailed with three(3) 12d Galv nails or two(2) 16d Galv nails at each end. Straps are not required on members less than 36" long.

13.) DESIGN LOAD PRESSURES FOR WINDOWS AND DOORS:

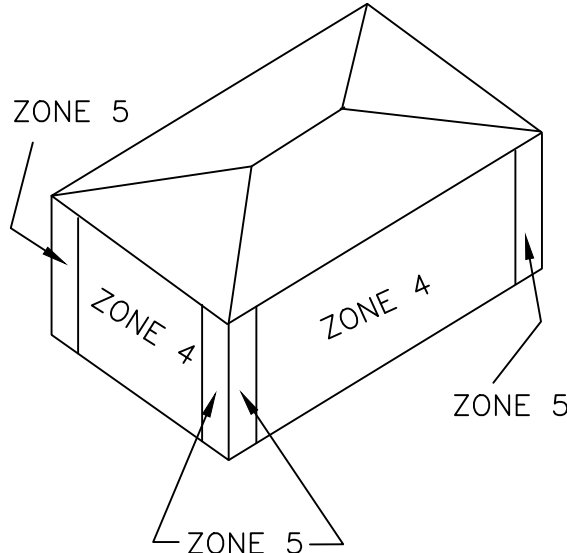
The table below is based on "ASD" (ALLOWABLE STRESS DESIGN) 2023 Florida Building Code, Building, Eighth Edition 1609.1.5, and assumes that the windows and doors have been tested in accordance with CHAPTER 17 of the 2023 Florida Building Code, Building, Eighth Edition and that the appropriate safety factor has been incorporated into their published load ratings. This engineering design is based on this building being an "enclosed" structure, therefore; All windows, sliding glass doors, and glass panels in doors shall be "impact resistant" meeting the minimum requirements of the 2023 Florida Building Code, Building, Eighth Edition (section 1609) for impact resistance or shall have "code compliant" protective shutters installed over the openings. All windows, sliding glass doors, and entry doors, shall be designed and certified by the manufacturer to resist minimum design wind pressures as shown below.

DESIGN PRESSURES for windows and doors

END ZONE AREAS (within 48" of corners) = ZONE 5

The pressures are tabulated by category for this residence. Positive pressures are pushing inward on the window/door; and negative pressures are pushing outward on the window/door.

"ALLOWABLE" WIND PRESSURES FOR COMPONENTS, CLADDING, DOORS, & WINDOWS WIND = 160 MPH – EXPOSURE C – MRH=<30" (POUNDS PER SQUARE FOOT)				
EFFECTIVE WIND AREA (SQ.FT.)	ZONE 4 (Away from corners)		ZONE 5 (Within 4' of corners)	
	POSITIVE	NEGATIVE	POSITIVE	NEGATIVE
0–19	33.4	–36.3	33.4	–44.7
20–49	31.9	–34.7	27.6	–41.7
50–99	29.9	–32.7	29.8	–37.7



14.) GARAGE DOORS – Commercial roll up garage doors are being used and shall be rated for +/-65.0 PSF in accordance with the 2023 Florida Building Code Eighth Addition. Garage doors shall be installed in accordance with the manufactures approved specifications for this wind load.

15.) TESTING & LABELING

All exterior windows and doors shall be tested and each product type certified to meet Florida Building Code FBC R 301.2.1.1, and the design pressures specified above, including attachment to the main structure. ALL GLAZED OPENINGS SHALL MEET THE REQUIREMENTS OF THE LARGE MISSILE TEST unless they are covered with "code compliant" protective shutters. All testing shall be in accordance with the testing provisions outlined in the 2023 Florida Building Code, Building, Eighth Edition. Product certification shall be provided to the Building Department by an independent testing laboratory meeting the qualifications outlined in the 2023 Florida Building Code, Building, Eighth Edition. The product certification shall include the minimum installation details (including fasteners and their spacing) required to resist the total force on the window when the design wind pressures are applied to the total area of the window. All Windows and glass doors shall bear an AAMA or WDMA or other approved label identifying the manufacturer, model or model number, performance characteristics, and approved product evaluation entity to indicate compliance with the requirements of ASTM E 1300, ANSI/AAMA/MWWD A 101/IS 2.

GENERAL NOTES

1.) DOORS & WINDOWS

All windows and exterior doors shall meet all the provisions of the 2023 Florida Building Code, Building, Eighth Edition including the wind load and impact criteria specified in the Engineering notes on sheet 1 and this sheet. Coordinate door styles with owner. Coordinate "rough" openings in new block walls with window and door manufacturers prior to construction.

2.) INSULATION

Install R20 "spray foam insulation" directly under the roof sheathing in the entire garage. This is the (minimum requirement) Coordinate with owner for additional insulation requirements.

3.) STUCCO

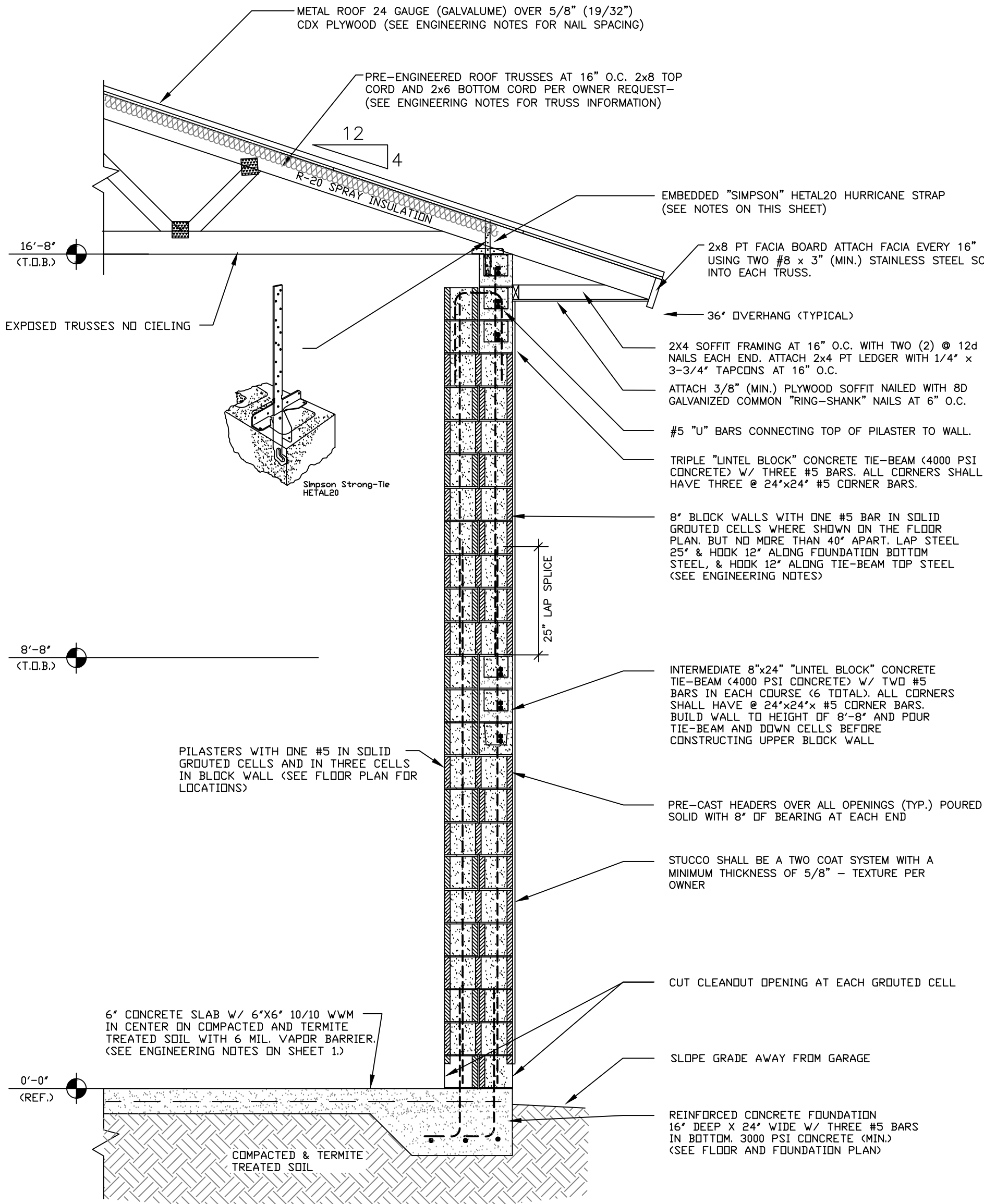
Stucco over masonry "stucco" block shall be a two coat system with a minimum thickness of 5/8". Always apply a bonding agent to all formed concrete and pre–cast lintels. Stucco materials & installation shall comply with 2023 Florida Building Code, Building, Eighth Edition.

4.) INTERIOR PAINT

The interior of the block walls shall have two coats of paint. Coordinate all textures, door styles, interior trim, and paint with owner.

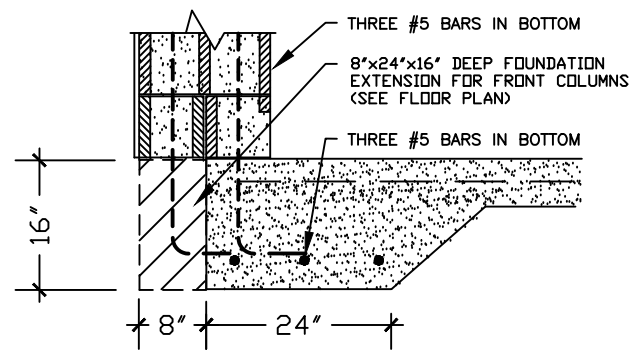
5.) EXTERIOR PAINT

Apply a minimum two coats of high quality exterior paint over primed surfaces. Coordinate paint type and color with owner. Apply paint in accordance with paint manufacturers specifications.

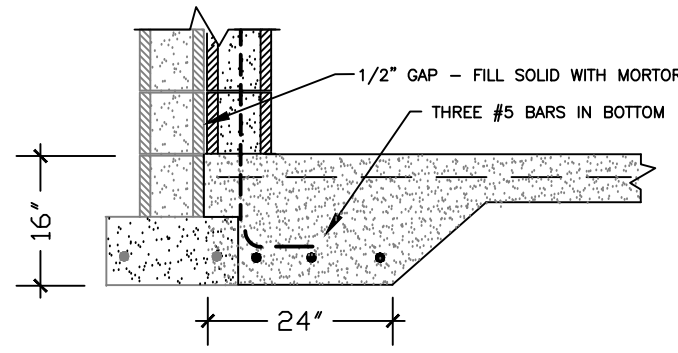


TYPICAL WALL SECTION

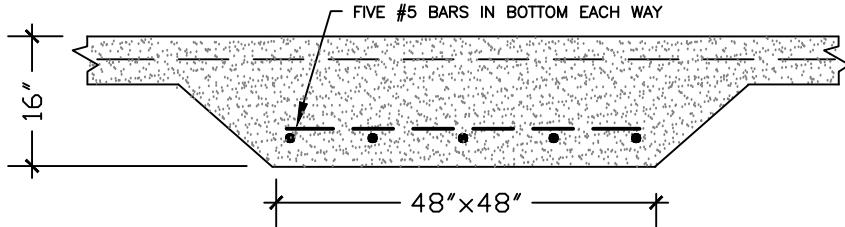
1/2"=1'-0"



F1 DETAIL N.T.S.



F2 DETAIL N.T.S.



F3 DETAIL N.T.S.

FOUNDATION DETAILS

1/2"=1'-0"

RELEASE/REVISION

SAMPLE
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This plan has been prepared under my direct supervision and is, to the best of my knowledge, accurate and in compliance with applicable building codes. This plan is for use only for this specific building at this location.

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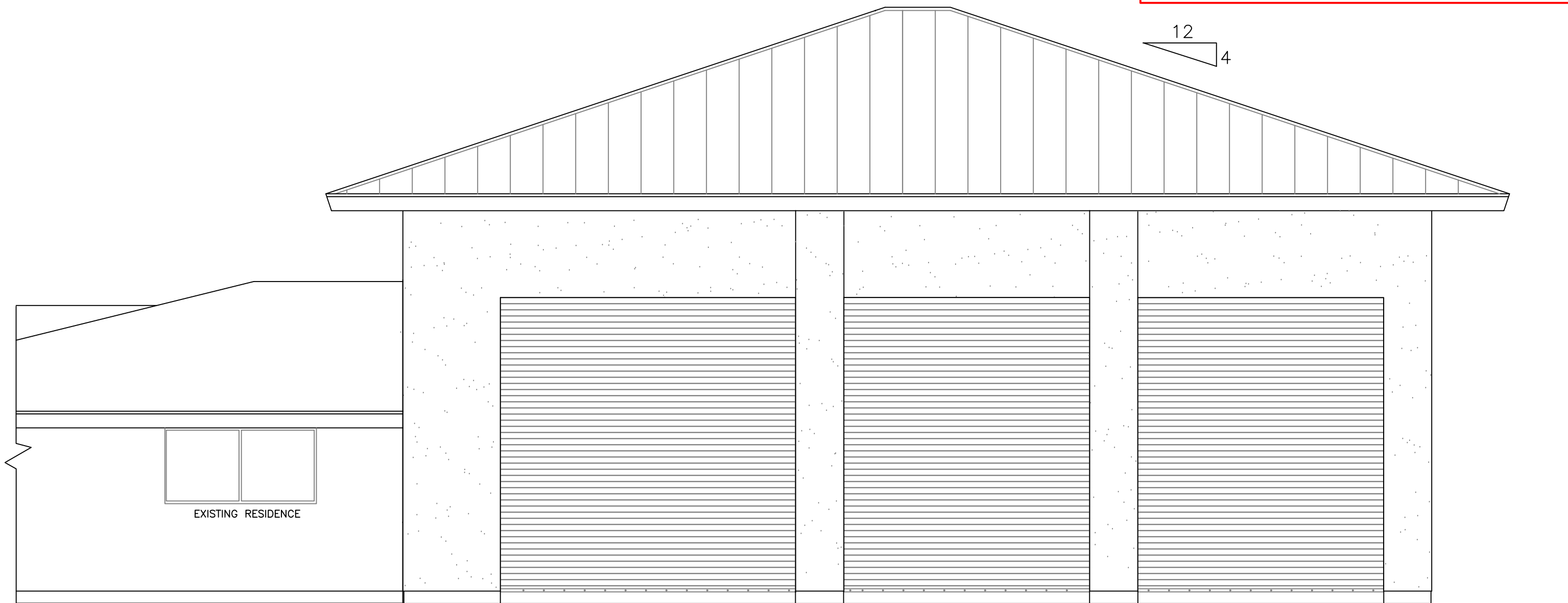
CUSTOM GARAGE DESIGN FOR:
NEW CLIENT
XXXX MAIN STREET
CITY NAME, FLORIDA 329XX

STRUCTURAL NOTES,
SECTIONS, & DETAILS

DRAWN MJS
DATE 06/16/26
CHECKED DLB
SCALE
JOB NO.
SHEET

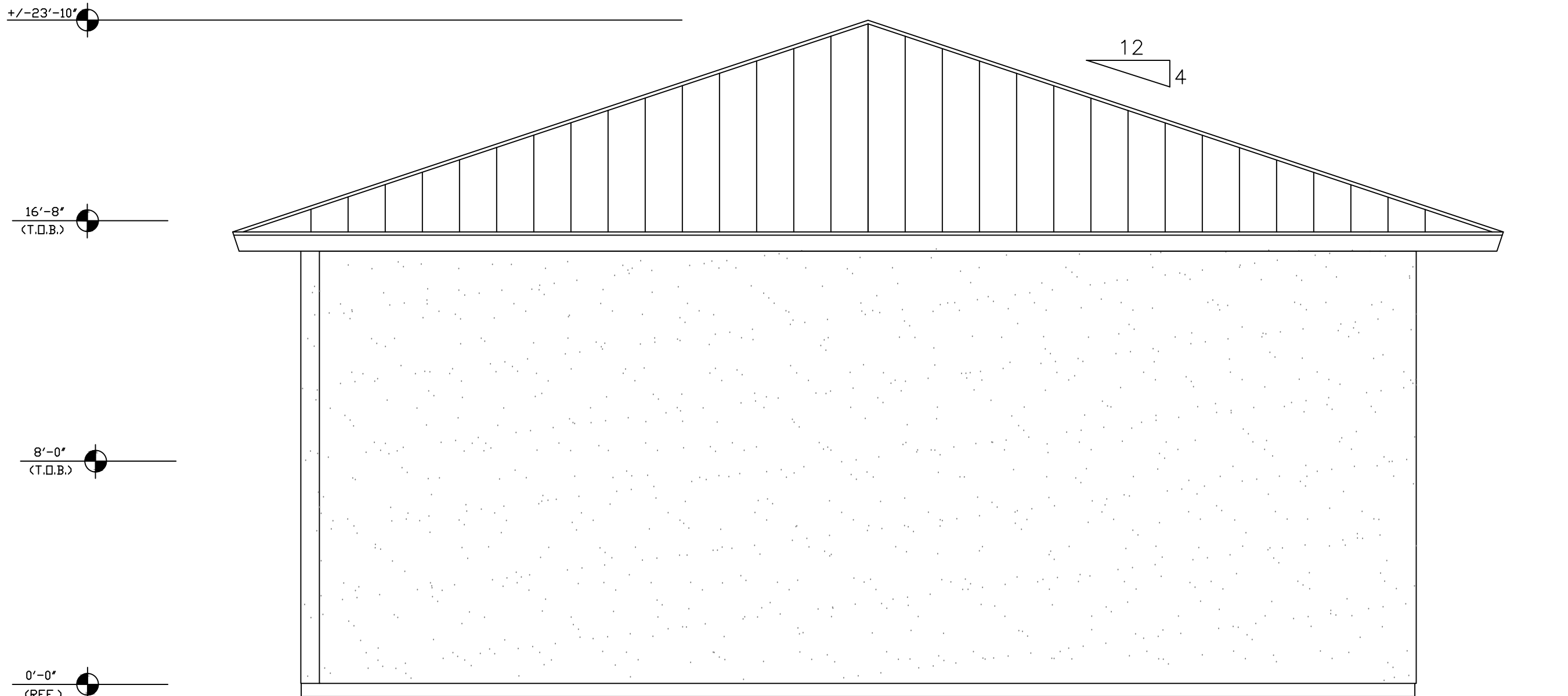
2
OF 6 SHEETS

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FRONT ELEVATION
SOUTH 1/4"=1'-0"

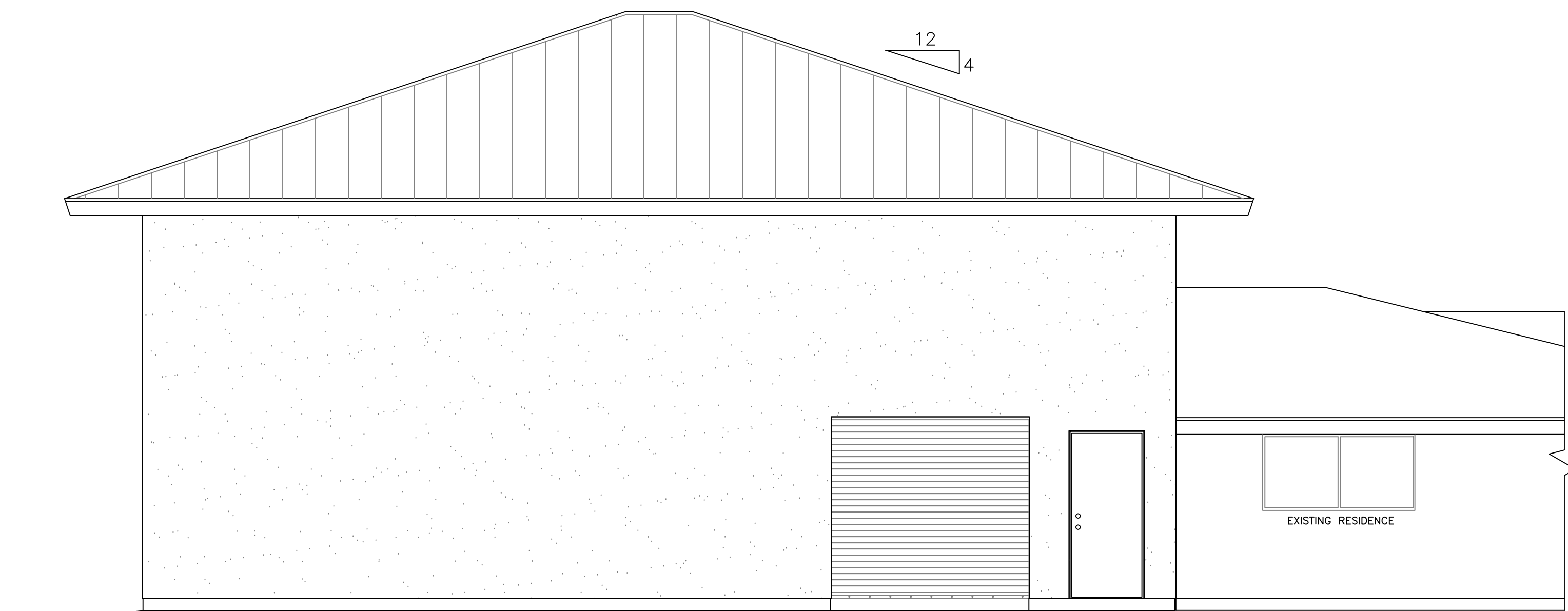
MAINTAIN SOIL 6" BELOW FFE
AND SLOPE AWAY FROM HOUSE



RIGHT SIDE ELEVATION
EAST 1/4"=1'-0"

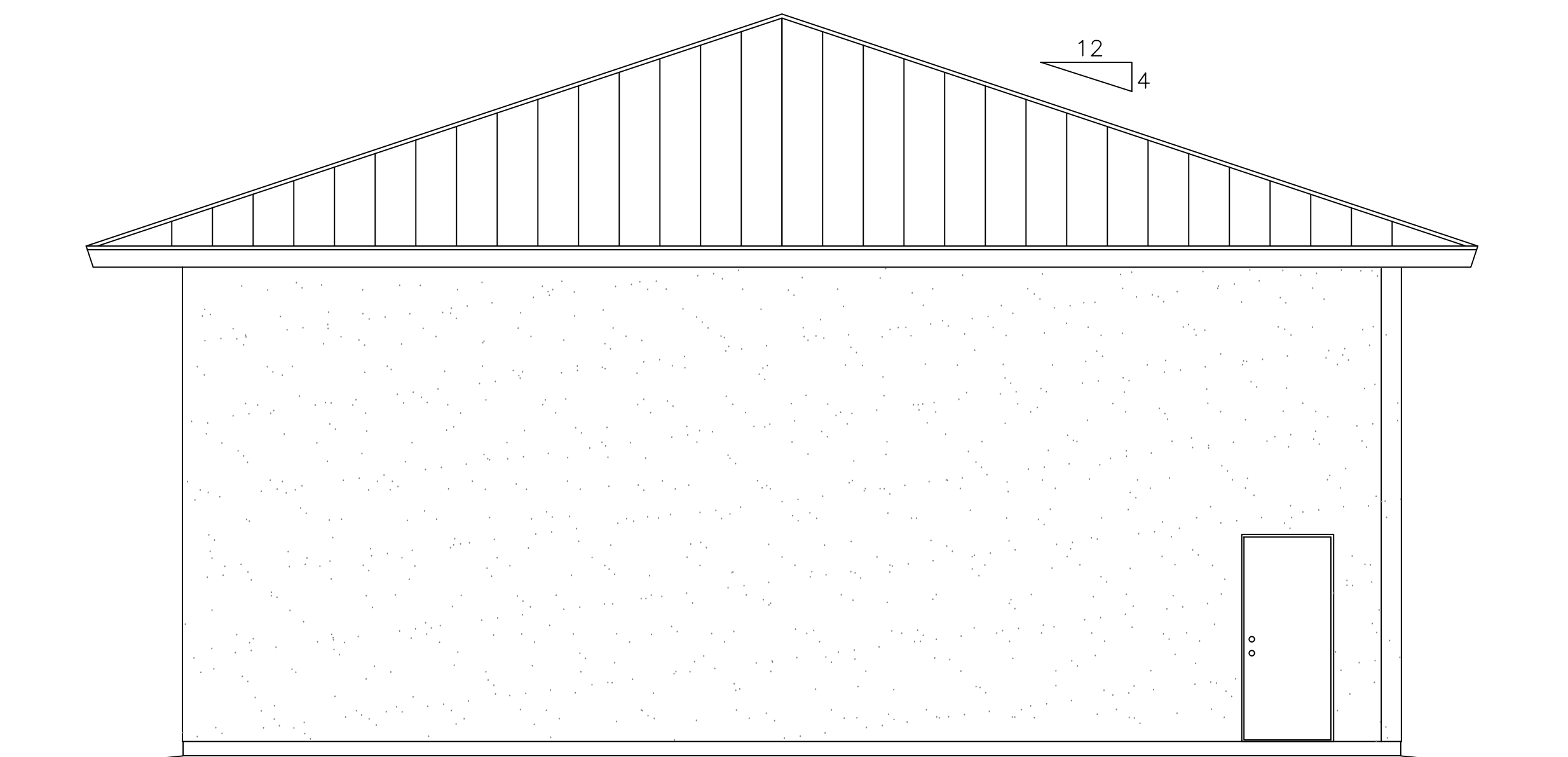
MAINTAIN SOIL 6" BELOW FFE
AND SLOPE AWAY FROM HOUSE

MAINTAIN SOIL 6" BELOW FFE
AND SLOPE AWAY FROM HOUSE



REAR ELEVATION
NORTH 1/4"=1'-0"

MAINTAIN SOIL 6" BELOW FFE
AND SLOPE AWAY FROM HOUSE



LEFT SIDE ELEVATION
WEST 1/4"=1'-0"

MAINTAIN SOIL 6" BELOW FFE
AND SLOPE AWAY FROM HOUSE

MAINTAIN SOIL 6" BELOW FFE
AND SLOPE AWAY FROM HOUSE

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CUSTOM GARAGE DESIGN FOR:
NEW CLIENT
XXXX MAIN STREET
CITY NAME, FLORIDA 329XX

ELEVATIONS

DRAWN	MJS
DATE	06/16/26
CHECKED	DLB
SCALE	
JOB NO.	
SHEET	3
OF	6 SHEETS

SAMPLE
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CUSTOM GARAGE DESIGN FOR:
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CITY NAME, FLORIDA 329XX

FLOOR, & FOUNDATION PLAN
AND ELECTRICAL PLAN

DRAWN	MJS
DATE	06/16/20
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SCALE	
JOB NO.	
SHEET	
	4
OF	6 SHEETS



- | ELECTRICAL LEGEND | |
|---|---|
|  | WEATHERPROOF RECEPTACLE
110V/15A ON GFCI CIRCUIT |
|  | 110V/15A GFCI CIRCUIT |
|  | 120V/20A DOOR OPERATOR |
|  | DOOR SWITCH |
|  | 3 POLE SWITCH |
|  | PHOTO CELL OUTDOOR LIGHT |
|  | 100A SUB PANEL |

STRUCTURAL INSPECTIONS

THE LOCAL BUILDING DEPARTMENT AND THE GENERAL CONTRACTOR SHALL VERIFY STEEL PLACEMENT PRIOR TO ALL CONCRETE POURS.

ALL REINFORCING STEEL SHALL BE INSPECTED FOR SIZE, QUANTITY, COVER, AND ADEQUATE BRACING TO RESIST MOVEMENT DURING CONCRETE PLACEMENT

CUT ONE BOTTOM FACE OUT OF ALL DOWNPOUR BLOCK CELLS (ABOVE SLAB LEVEL), WITH A MINIMUM AREA OF 12 SQ. INCHES AND A MINIMUM DIMENSION OF 3 INCHES, TO CLEAN CELL, INSPECT STEEL, AND VERIFY A PROPER POUR.

EXISTING RESIDENCE	2882 SF
GARAGE	1652 SF
TOTAL SQUARE FOOTAGE	4534 SF

DIMENSIONS, DISCREPENCIES, AND ELEVATIONS

CONTRACTOR SHALL VERIFY ALL DIMENSIONS PRIOR TO BEGINNING ANY WORK. ANY SIGNIFICANT DISCREPANCIES SHALL BE DISCUSSED WITH THE ENGINEER PRIOR TO ORDERING ANY TRUSSES AND PRIOR TO POURING ANY FOUNDATIONS.

DIMENSIONS SHOWN ARE "ROUGH DIMENSIONS" AND DO NOT INCLUDE DRYWALL OR FINISHES.

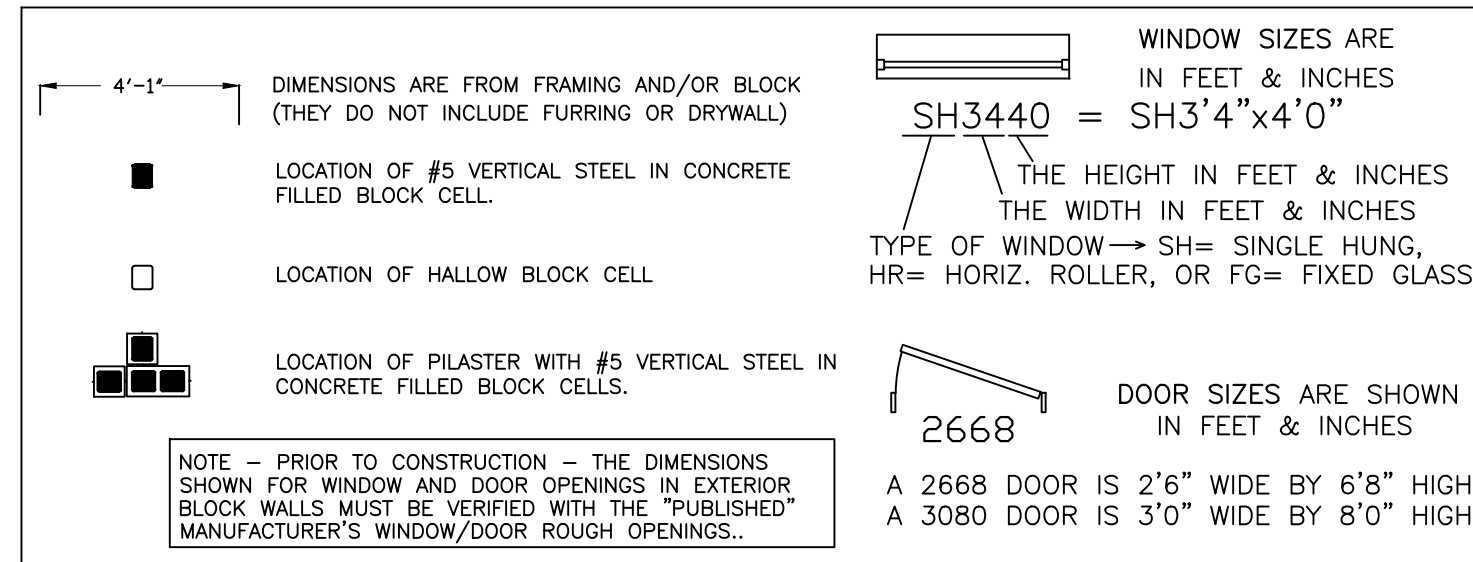
6" THICK 3000 PSI CONCRETE WITH 6"x6" 10/10 WWM IN THE CENTER, POURED OVER 6 MIL. VAPOR BARRIER ON TERMITE TREATED COMPACTED AND TESTED SOIL. (SEE SHEET 2)

SAWCUT CONTROL JOINTS IN THE SLAB WITHIN 24 HOURS OF POURING.
SAWCUTS SHALL BE 1-1/2' DEEP AND SPACED NO MORE THAN 10 FEET APART
IN EACH DIRECTION

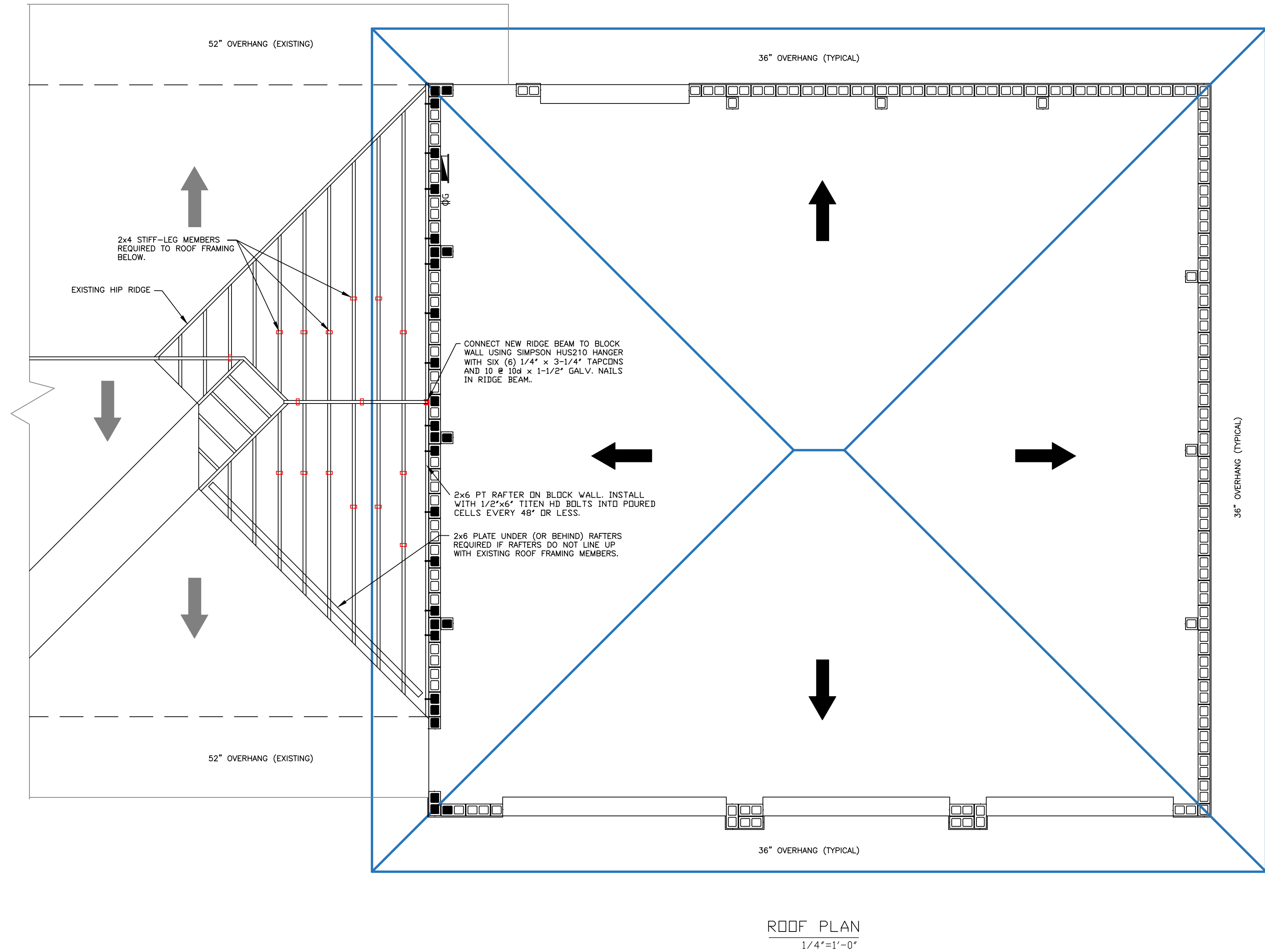
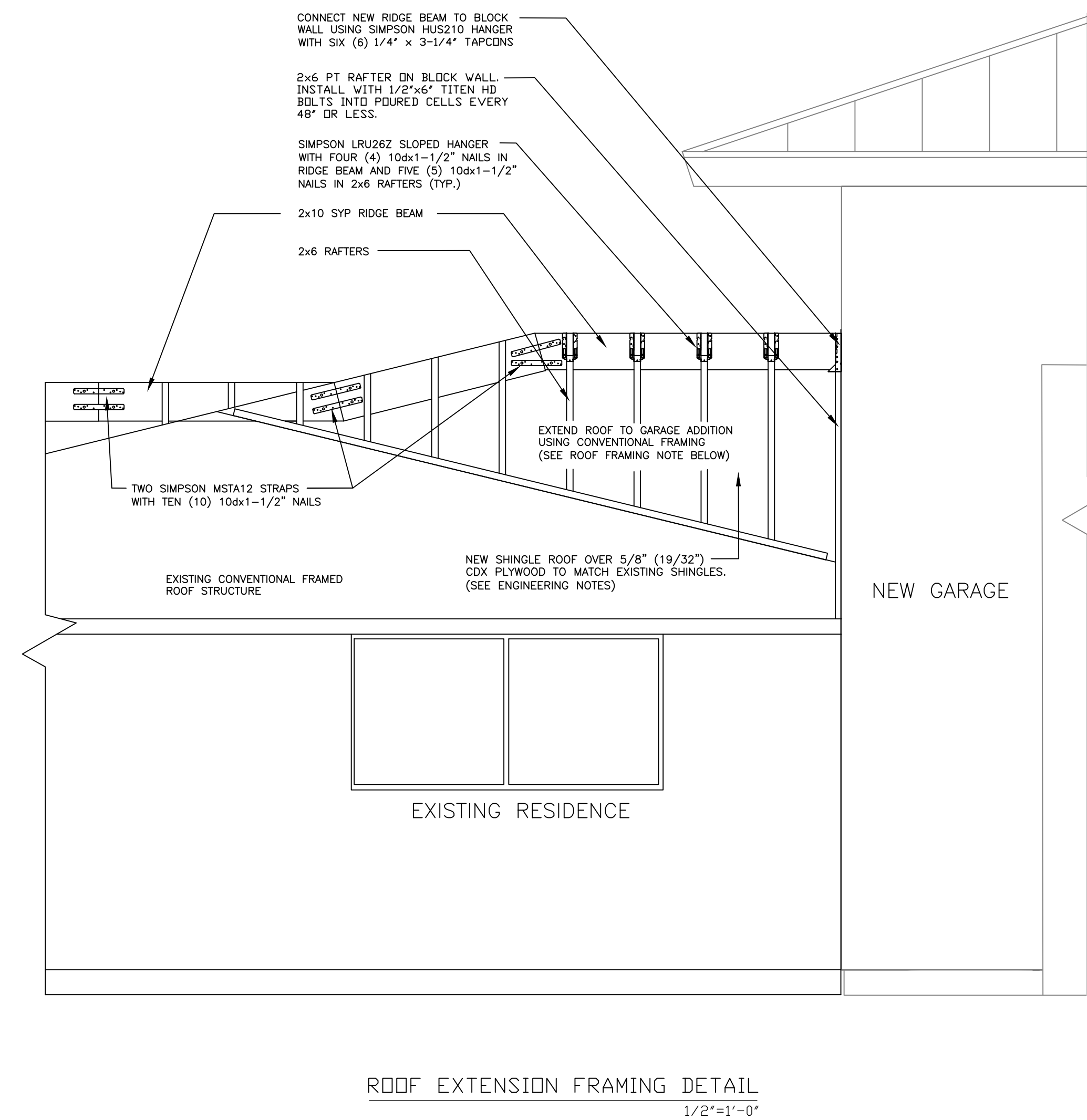


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LEGEND



IF REQUIRED BY THE BUILDING DEPARTMENT,
A FOUNDATION SURVEY SHALL BE PERFORMED AND THE SIGNED & A SEALED SURVEY
SHALL BE SUBMITTED AND APPROVED BY THE BUILDING DEPT. PRIOR TO THE SLAB
PRE-POUR INSPECTION.



CONVENTIONAL ROOF FRAMING TO EXTEND THE ROOF ON THE EXISTING RESIDENCE
Where conventional framing is used to "fill-in" and extend the existing roof, a new 2x10 SYP Ridge Beam shall be installed and connected to the existing ridge beam using 5/8" plywood or metal plates on each side. Connect Ridge Beam to new garage wall using a SIMPSON HUS210 Hanger. Install 2x6 SYP rafters at 16" on center. Connect each Rafter to new ridge beam using a SIMPSON LRU26Z Sloped Hangers.

The new Ridge Beam and the new Rafters shall ALSO be support with 2x4 vertical "Stiff-Legs" members every 6 feet or less. The Stiff-Legs shall line up over existing roof framing members and be strapped at each end using SIMPSON LSTA 12 Straps.

On all members over 36" long – Install HURRICANE STRAPS

Attach base of rafters and each end of other framing members to resist uplift forces equal to 50 lbs. multiplied by the overall rafter/member span length (from stiff-leg).
i.e – A 6'0" long framing member requires a strap rated for 300 lbs. Members shorter than 36" can be nailed with three(3) 12d Galv nails or two(2) 16d Galv nails at each end. Straps are not required

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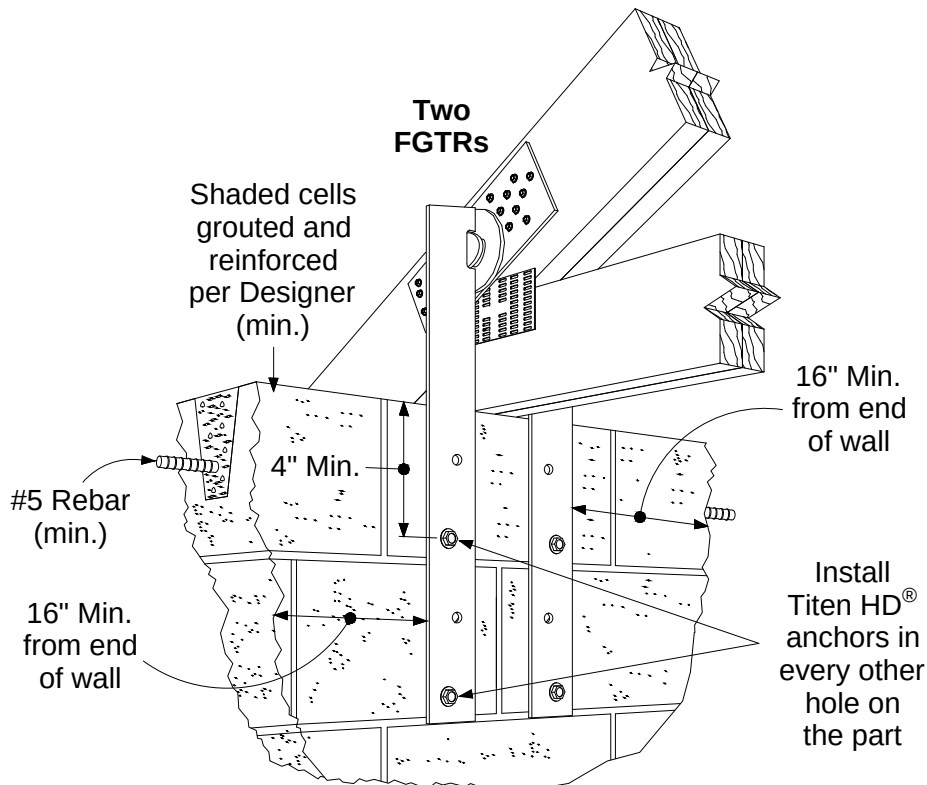
CUSTOM GARAGE DESIGN FOR:
NEW CLIENT
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ROOF PLAN
& DETAILS

DRAWN MJS
DATE 06/16/26
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SCALE
JOB NO.
SHEET
5
OF 6 SHEETS

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TRUSS "HURRICANE" CONNECTOR SCHEDULE	
TRUSS TYPE	HURRICANE CONNECTORS REQUIRED (BLOCK WALLS WITH TIE-BEAMS)
SINGLE TRUSS (1-PLY) SUPPORTED BY TIE-BEAM	Up to 1810 lbs uplift – must be single ply truss – One "Simpson" HETAL20 embedded truss straps 6" into the tie beam, with (14) – 10d x 1-1/2" galv. nails.
DOUBLE TRUSS (2-PLY) SUPPORTED BY TIE-BEAM	Up to 2500 lbs uplift – must be double ply truss – Two (2) "Simpson" HETAL20 embedded truss straps 6" into the tie beam, with (14) – 10d x 1-1/2" galv. nails. in each.
TRIPLE TRUSS (3-PLY) SUPPORTED BY TIE-BEAM	Up to 8885 lbs uplift – must be triple ply truss – Two (2) "Simpson" FGTR girder tie-downs bolted into the tie beam, with four 1/2" x 5" Titen HD Bolts and thirty six 1/4" x 3" SDS screws.
TRUSS TO TRUSS CONNECTORS	All truss to truss connectors (hangers) shall be specified by the truss engineer and on the truss engineering layout sheet and provided by the truss manufacturer.



SIMPSON FGTR GIRDER TRUSS CONNECTORS
FOUR LOCATIONS N.T.S.

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4280 DOW ROAD, SUITE 108
MELBOURNE, FLORIDA 32934
(321) 254-7913

This plan has been prepared under my direct supervision and is, to the best of my knowledge, accurate and in compliance with applicable building codes. This plan is for use only for this specific building at this location.

CUSTOM GARAGE DESIGN FOR:
NEW CLIENT
XXXX MAIN STREET
CITY NAME, FLORIDA 329XX

TRUSS PLAN

DRAWN	MJS
DATE	06/16/26
CHECKED	DLB
SCALE	
JOB NO.	
SHEET	6
OF	6 SHEETS